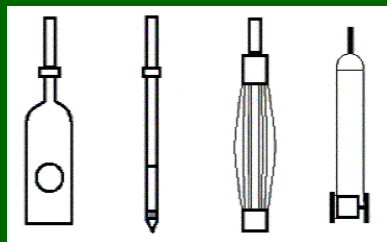


Use of Rock Pressuremeter for Deep Foundation Design



Roger A. Failmezger, P.E.



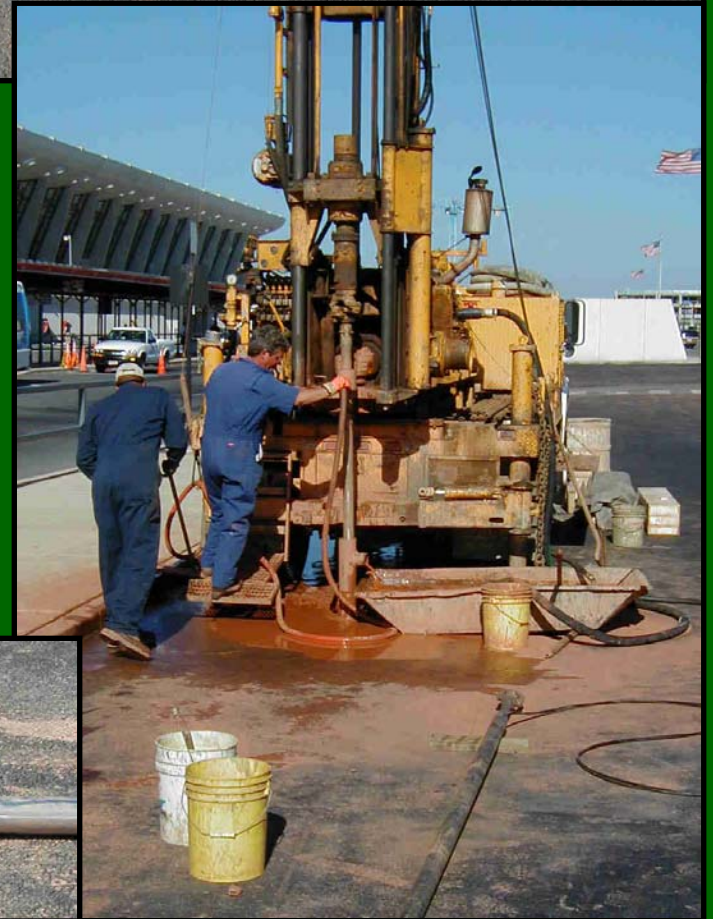
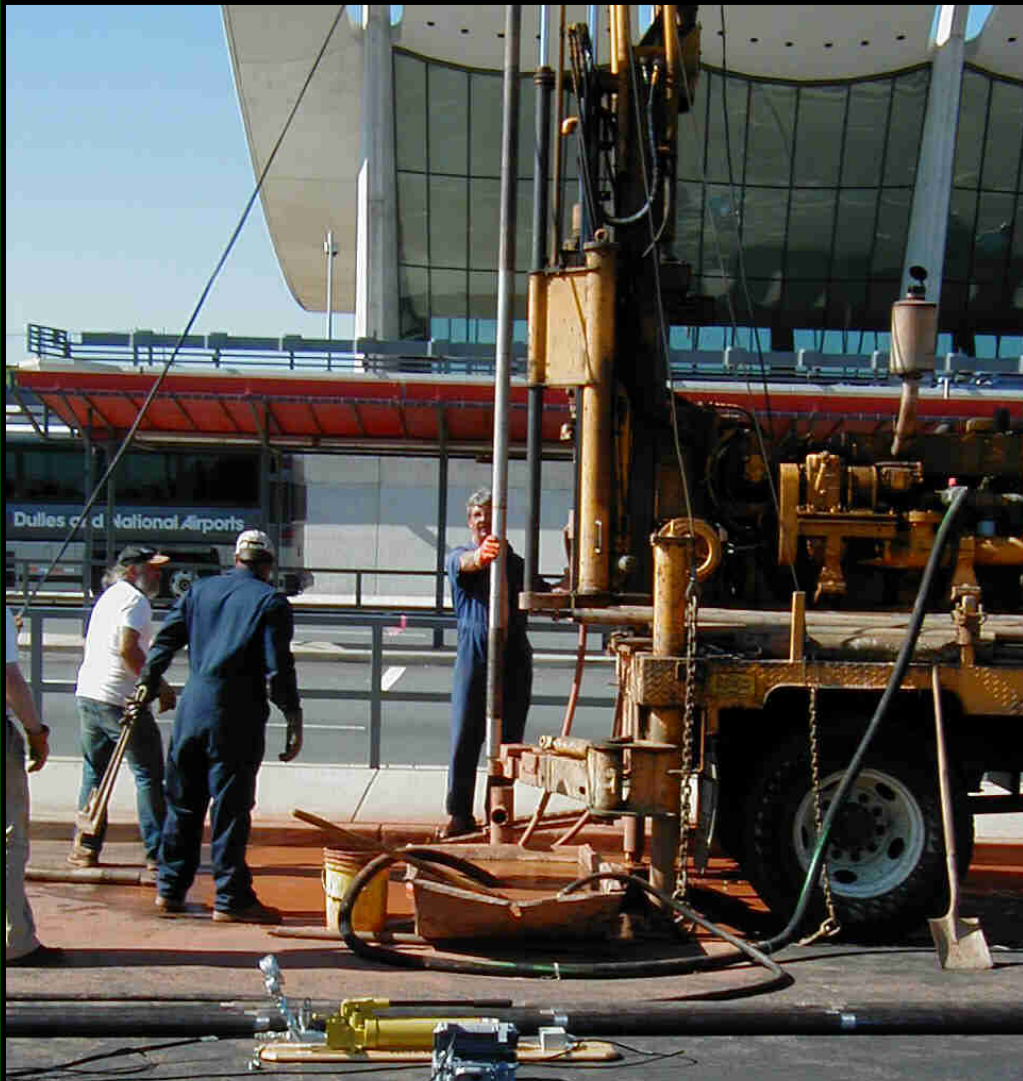
Aaron L. Zdinak, P.E.
Jesse N. Darden, P.E.

Randall Fahs

HILLIS-CARNES

Rock Pressuremeter Test

- ⊕ Pressure applied by pumping hydraulic jack and extending piston downward inside probe
- ⊕ Piston pressurizes ethanol glycol-water mixture inside membrane causing it to expand
- ⊕ Piston displacement is measured with an LVDT
- ⊕ Volume is computed as piston displacement times piston area
- ⊕ Pressure is measured at surface with transducer



Rock PMT Advantages

- ⊕ Can apply 30 MPa of pressure
- ⊕ Because volume is measured inside probe eliminate correction for tubing
- ⊕ Can obtain accurate modulus values
- ⊕ Can obtain limit pressure in softer rock
- ⊕ Membranes are robust

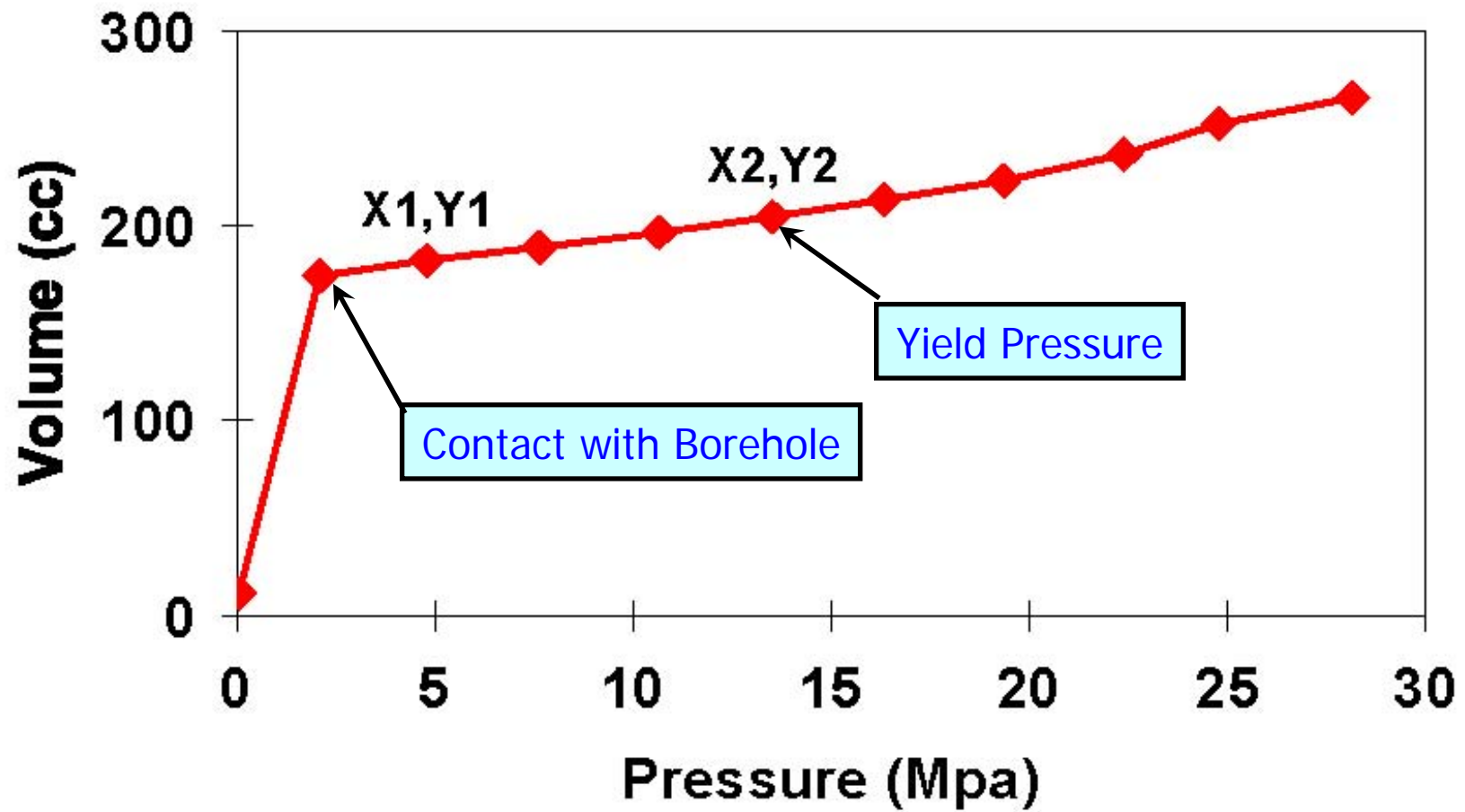
Rock PMT Disadvantages

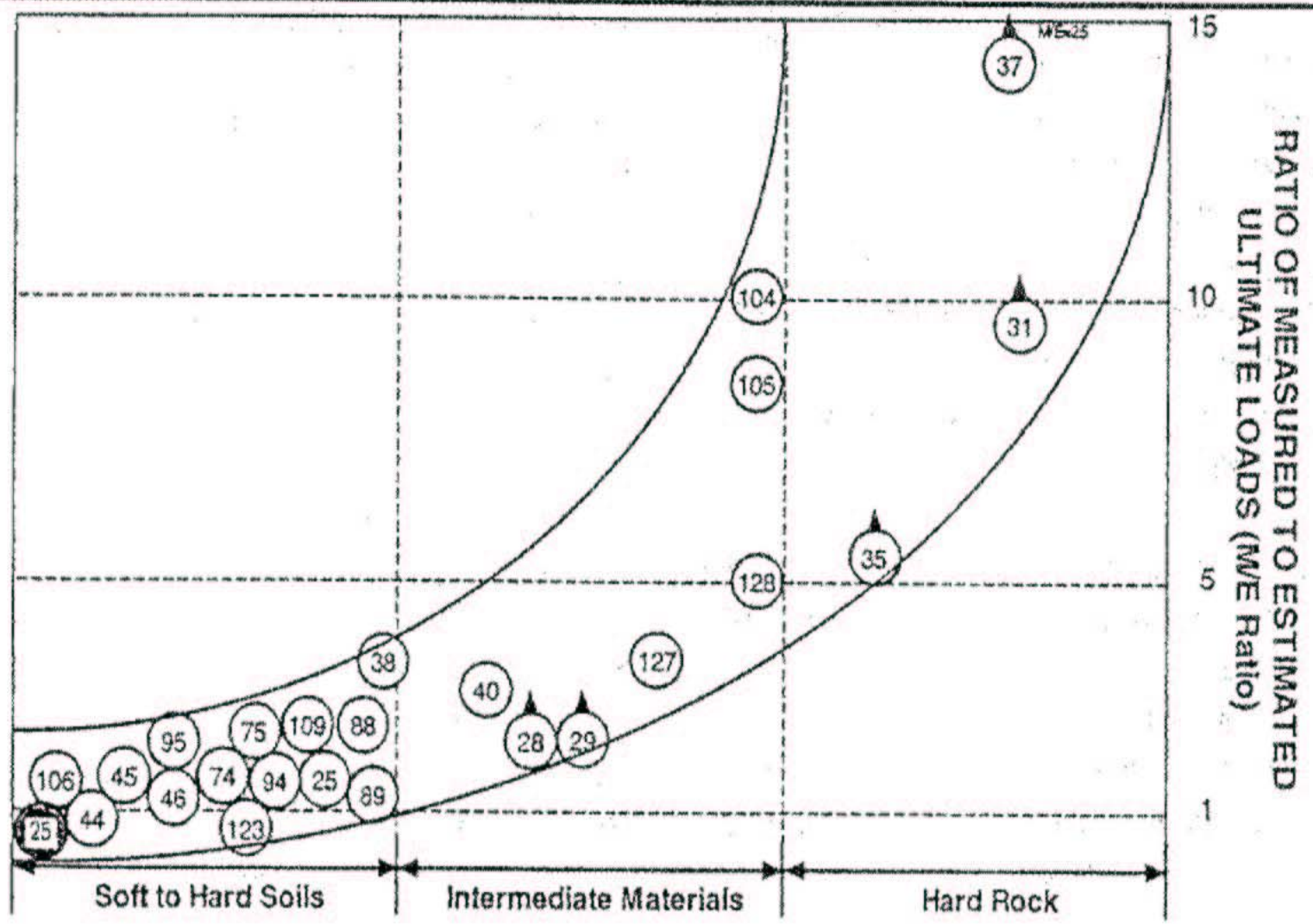
- ⊕ Can only practically expand membrane about 300 cm³
- ⊕ Clay filled rock seams can rupture membranes
- ⊕ Membranes are costly (about \$2500 U.S.)



Typical Calibration Values

- ⊕ System Compressibility: $\sim 1 \text{ cm}^3/\text{MPa}$
- ⊕ Membrane Resistance: $\sim 2.5 \text{ MPa}$ at 400 cm^3

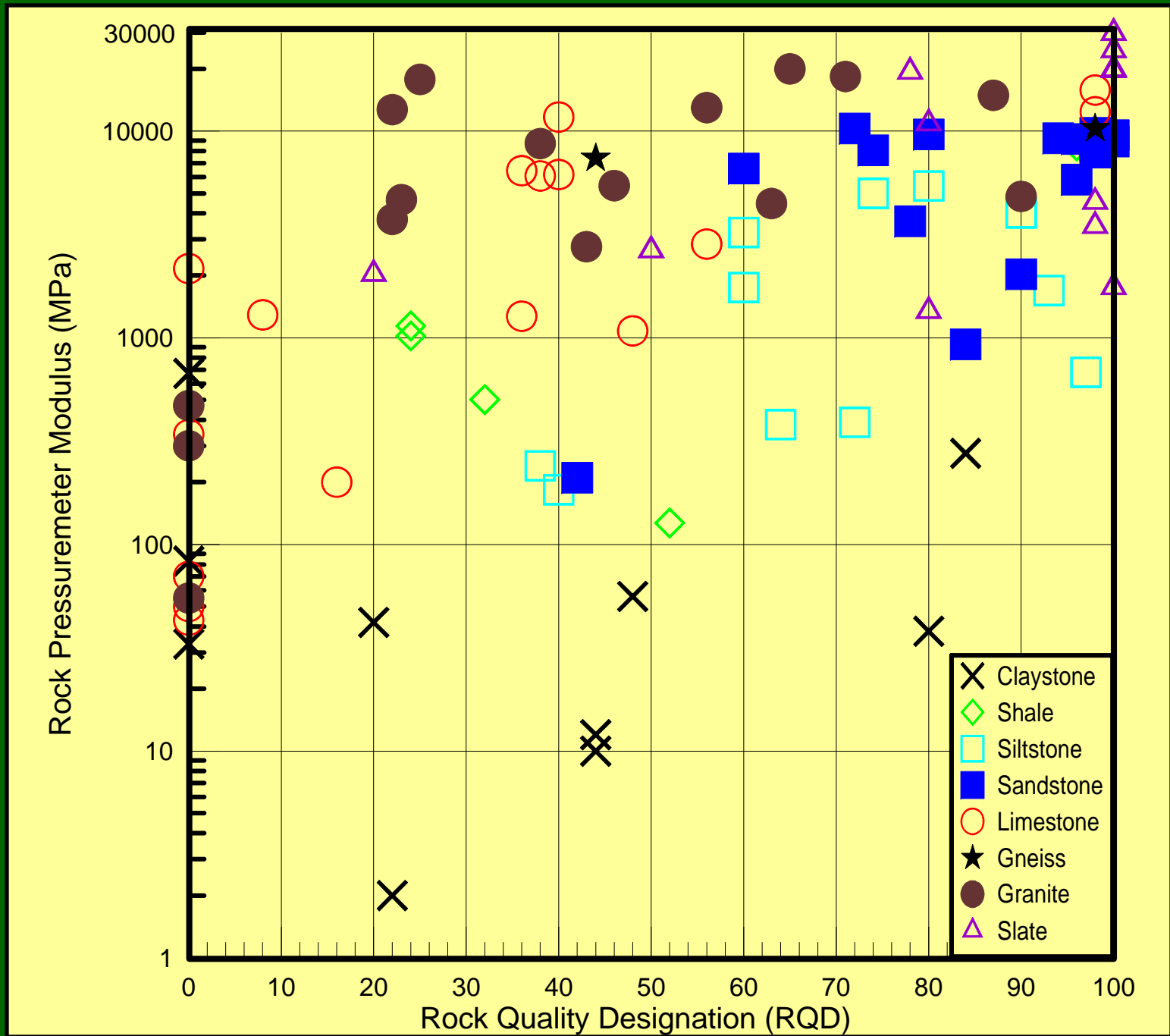
Volume vs Pressure

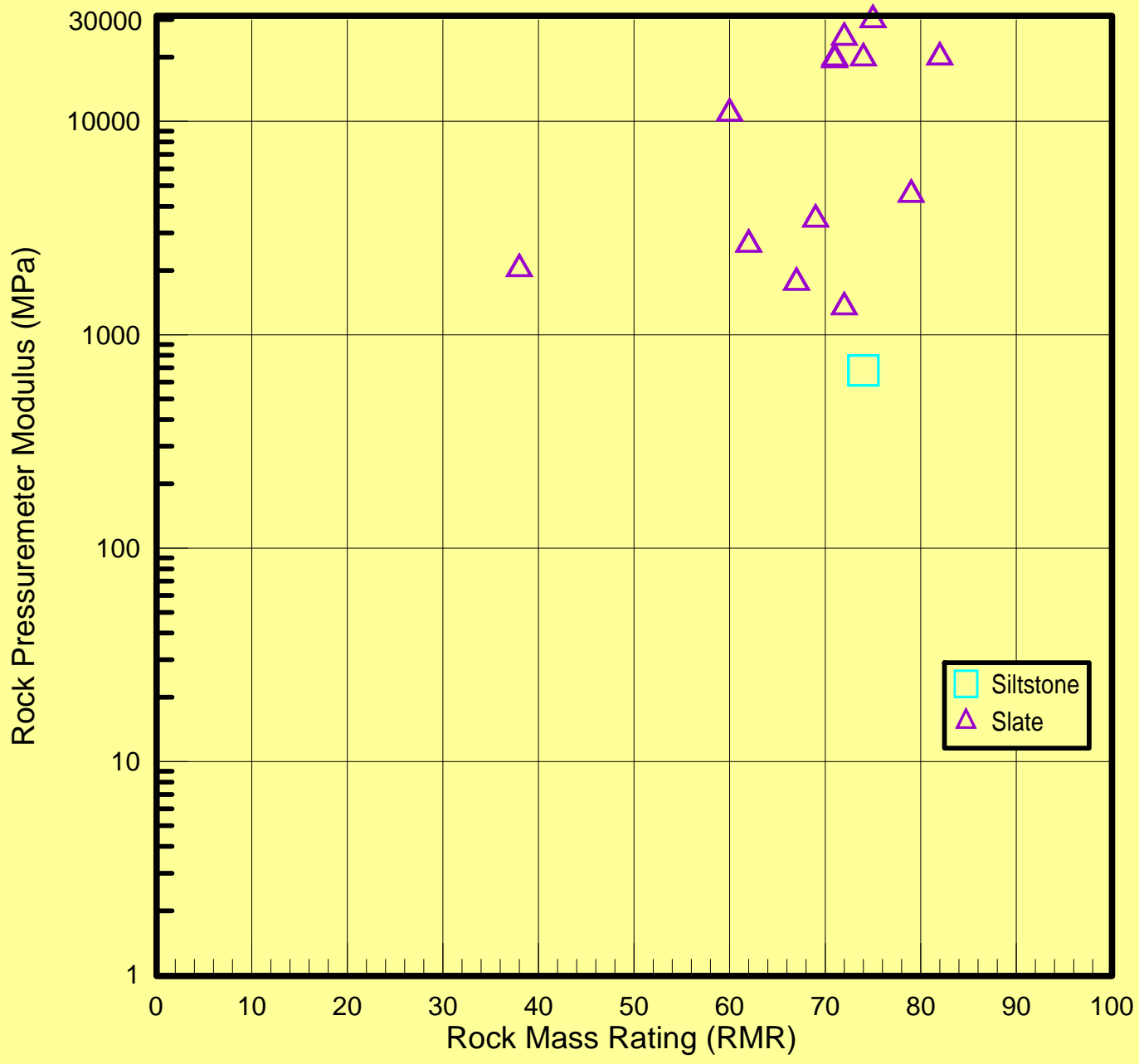




- The denoted numbers are LOADTEST Inc. file references.
-  Indicates neither component reached an ultimate, i.e. M/E would be much higher than shown
 -  Indicates low results due to poor construction technique.

Schmertmann and Hayes, 1997





Lateral load design procedure (Modified from Robertson, et al., 1985)

- ⊕ Re-interpret pressuremeter results in terms of radial strain and pressure
- ⊕ Generate P-y curves
 - ⊕ Multiply radial strain component by pile half-width
 - ⊕ Multiply pressure component by pile width
 - ⊕ Do not reduce pressure component by an α -factor

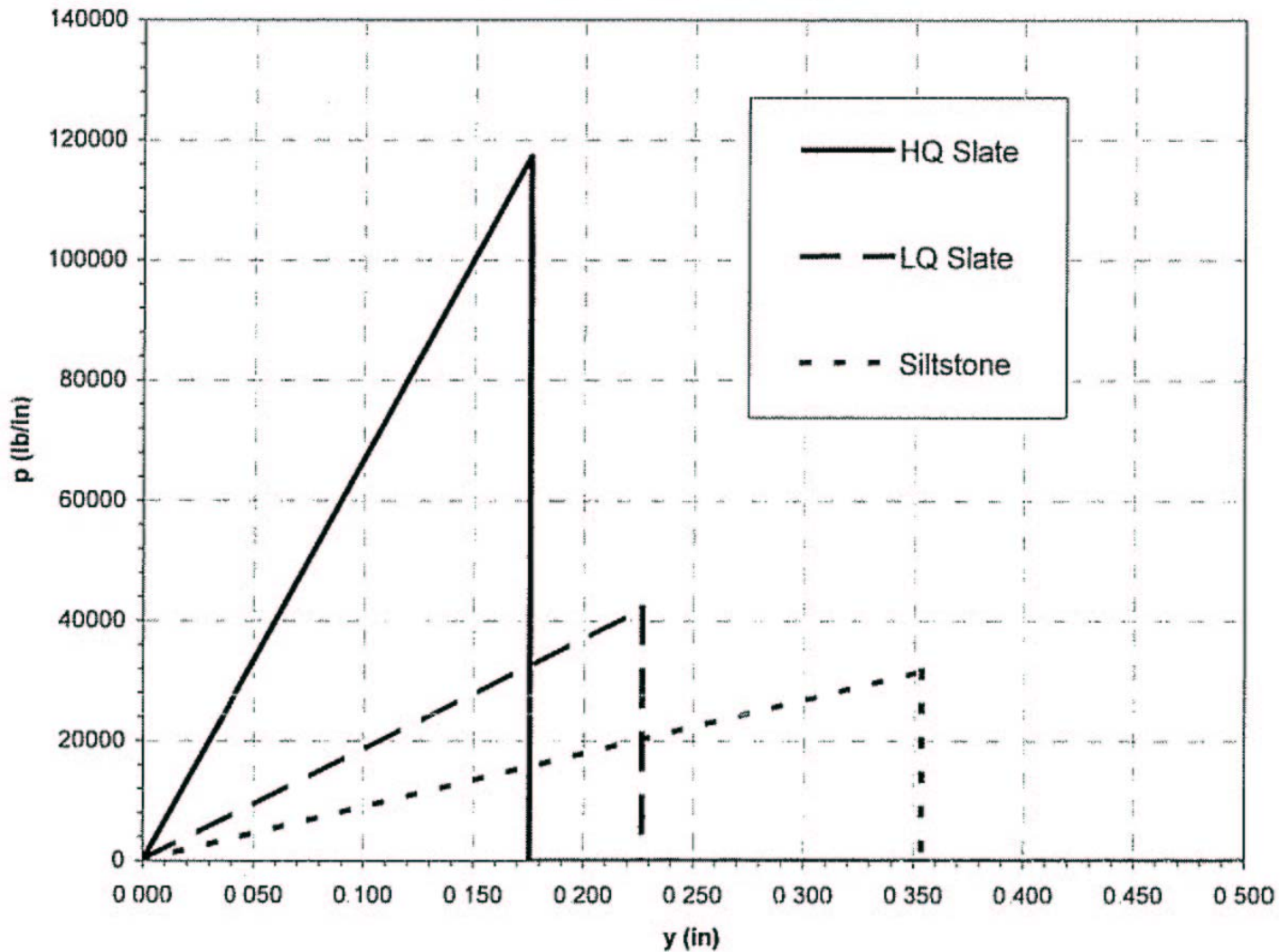
Lateral analysis (cont.) (Proposed Procedure)

- ⊕ Compute the average slope and its standard deviation for each rock group
- ⊕ Set the design slope equal to the average slope minus 2 standard deviations
- ⊕ Apply a “limiting strain” value to account for brittle behavior
 - ⊕ Set value equal to PMT yield pressure

Case Study—I-95 Occoquan River

Rock Classification	Rock Mass Modulus Range (MPa)	Average RQD	Average UC (MPa)	Average RMR	Comments
High Quality Slate	$E_m > 10,000$	94	60	74	Slate bedrock, fractures infrequent
Low Quality Slate	$1000 < E_m < 10,000$	78	26	64	Slate bedrock, fractures, brecciated zones, and slickensides more frequent, loss of water return common during drilling
Siltstone	$E_m < 1000$	92	43	74	Siltstone bedrock

Design P-y Curves



Conclusions

- ✦ Rock PMT Modulus values improve lateral load design of drilled shafts in rock
- ✦ Rock PMT modulus are representative of “rock mass modulus” and do not need reduction corrections
- ✦ Rock is quite heterogeneous—Modulus design values should be average value minus “x” standard deviations—we chose 2 S.D.

Conclusions (cont.)

- ⊕ Rock type is as important to modulus as RQD or RMR
- ⊕ No meaningful correlation found between rock modulus and RQD
- ⊕ Correlation may be possible between rock modulus and RMR—but our data was limited

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